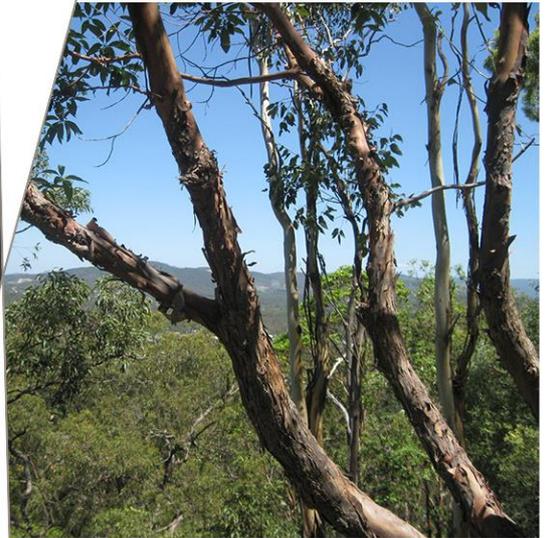


Preliminary Engineering Report

Scenic Rim Agricultural Industrial
Precinct

5103/57-001



Prepared for
Kalfresh Pty Ltd

25 September 2023

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Executive Summary

Kalfresh Pty Ltd has commissioned Stantec (Qld) Pty Ltd (Stantec) to provide preliminary civil engineering advice to support the proposed Scenic Rim Agricultural Industrial Precinct (SRAIP) Development Approval application to create an agricultural industrial precinct at 6200-6206 Cunningham Highway, Kalbar, Scenic Rim. The proposed development is referenced as 'The Scenic Rim Agricultural Industrial Precinct (SRAIP)' and intends to create a formal hub for fresh and frozen food production in an agricultural region 84km south-west of Brisbane.

This report includes recommendations for the civil works required to create a developable land formation for the proposed industrial allotments and road corridors. The civil works proposed to facilitate the development includes bulk earthworks, roadworks, stormwater drainage, stormwater quantity management, stormwater quality management, water reticulation and sewerage infrastructure.

This report also includes preliminary stormwater calculations in relation to the composting facilities to the west of the main industrial development.

The preliminary engineering recommendations and design considerations documented by Stantec are summarised in this report.

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The proposed development area topography is relatively flat with a gradual fall towards the northern boundary at a grade of circa 1:300 (0.33%). The maximum ground elevation is 84.0m AHD at the southern boundary whilst the minimum ground elevation is 81.0m AHD at the northern boundary. There is an existing overland flow path running along the western extents of the proposed development area which will be reconfigured to cater for the 1% AEP flood event as part of the proposed works. The land towards the rear of the property, outside of the development footprint and on the opposite side of the overland flow path, rises sharply towards the western boundary.

Figure 1-2 below provides an overview of the proposed development area.

Figure 1-2 Development Site Area



2 Civil Works

2.1 Clearing & Site Preparation

The site development area currently consists of agricultural cropping land and the existing Kalfresh production facilities. The development of this site necessitates that the area is cleared completely of any existing vegetation including the removal of any unsuitable material identified prior to the commencement of bulk earthworks and civil works construction.

It is recommended that the early establishment of erosion and sediment control measures is carried out in conjunction with the commencement of clearing works in order to prevent the discharge of soils from the site and protect existing downstream infrastructure.

2.2 Earthworks

Bulk earthworks will be completed across the subject site to create a developable land formation in accordance with Stantec sketch 510357-001-CI-1010. This earthwork operation will include the stripping/ stockpiling of topsoil and reshaping of land to generally achieve the proposed site levels across the development. In addition, Stantec have prepared a bulk earthworks engineering set of drawings which are also included in this application.

Based upon Stantec's experience within the Scenic Rim region and surrounding areas, coupled with the elevated site levels, it has been assumed that the site is devoid of acid sulphate soils and contaminated land.

The preliminary earthworks design comprises of filling the site above the 1% AEP flood level (plus climate change allowance) with an additional 300mm freeboard provided. This will require additional material which is proposed to be obtained from the deepening of the flood diversion channel and from within on-site borrow pits. Hence, the haulage distance of fill material will be limited to within the existing allotment boundaries and haulage of material from outside the subject site will be minimal.

The earthworks design based upon the proposed development layout indicates that the earthworks operation will comprise approximately 489,000m³ cut to fill onsite and 217,000m³ of additional fill required which will be obtained from borrow pits within the subject site (locations indicated on drawing 510357-001-CI-1010).

The proposed earthworks profile has been created with the intent to minimize the amount of fill whilst ensuring the development can be appropriately serviced by a stormwater drainage network and also be resilient to the 1% AEP flood event.

All earthworks on the site will be carried out in accordance with Level 1 supervision and testing requirements, with any existing dams and/or unsound materials being removed and replaced under Level 1 supervision.

It is also recommended that prior to the de-commissioning of all sediment basins, all collected silt and unsuitable material should be removed from the site and the basin area rehabilitated using a high quality of fill material in order to ensure long-term stability to this area of the site.

The Stantec sketch 510357-001-CI-1010 included in **Appendix B** of this replot illustrates the proposed cut and fill zones.

2.3 Erosion & Sediment Control

In accordance with IECA Best Practise Guidelines and Scenic Rim Council standards, it is proposed that in conjunction with the Operational Works Application phase of development a detailed Soil Erosion and Sediment Control Plan will be prepared in order to address the specific measures to be implemented manage erosion onsite and limit sediment discharge offsite. During the construction phase, the Contractor is to have a certified erosion and sediment control plan on site at all times.

General measures to be implemented during the construction phase in order to prevent the erosion of sediment from the site are as follows:

- Contractor to achieve temporary, interim or permanent ground cover to disturbed earthworks areas as soon as practicable;
- Sediment filter fencing is to be located at the downstream end of all open earthworks to remove sediment from overland flow prior to discharging off site;
- Truck shake down areas shall be provided to remove any loose materials from vehicles prior to departure from the site;

-
- All sediment control structures must be maintained in an effective operational condition. These structures must not be allowed to accumulate sediment volumes in excess of forty percent of the sediment storage design capacity;
 - If topsoil will be stripped and stockpiled, perimeter silt fences are to be installed around the stockpile areas to prevent the material discharging from the site;
 - All sediment control structures are to be supplied and installed in accordance with Scenic Rim Regional Council planning scheme policies and IECA Best Practice Guidelines; and
 - A sediment pond is to be constructed to suit the construction site profile and sized appropriately to capture the require volume of sediment laden runoff.

2.4 Roadworks

An internal road network will be created in order to provide access to each of the proposed industrial allotments and access and egress from the overall development. There are two proposed road profiles within the development, including:

- Road 1: consists of a 30.3m wide road reserve with an 14m carriageway which consists of 3.5m traffic lane and 3.5m parking bays.
- Road 2: consists of a 40m wide road reserve with 2 x 8m wide carriageways on either side of a 15m wide grass swale. The introduction of the grass swale between the 7m carriageways will used to capture major stormwater flows from the adjoining road and lots. The 8.0m carriageways consist of a 3.5m traffic lane, a 3.5m parking bay and a 1m shoulder adjacent the swale batter. 4.5m wide verges with 1.5m wide pathways are provided on each side.

These sections reflect a modified version of an 'Industrial Collector Street' profile as shown in Figure 9.7.A of IMEAQ Queensland Streets.

The main collector street traverses the centre of the site with a cul-de-sac provided at the south-west of the alignment to facilitate turning movements. This layout provides access to each lot whilst ensuring sufficient manoeuvrability for Class 10 (B-double) trucks within the industrial estate.

Provision for a future road connection towards the north-west of the site has also been facilitated within the layout. The proposed road will traverse the waterway channel via a weir structure. The design intention of the weir is to (a) allow safe crossing of the overland flow channel by vehicles accessing the north-west of the subject site, and (b) act as a flood mitigation device which restricts regional flows within the channel below the 10% AEP level. The weir will be constructed with 2 x 2400(W) x 300(H) RCBC culverts. These will be set at the IL of the waterway diversion channel to allow low flows within the channel to free-drain. A section of the weir is provided on Stantec sketch 510357-001-CI-1300.

Access to the development will be provided by a new intersection onto the Cunningham Highway located circa 460 m to the north-east of the existing access point. The intersection layout and geometry will be designed to TMR standards and will accommodate Class 10 (B-double) vehicles. A right-turning lane will be provided on the Cunningham Highway for vehicles entering the site. Within the site, no direct lot access will be permitted within 50 metres of the new Cunningham Highway intersection.

The 3 No. existing access points on to the Cunningham Highway currently utilised by Kalfresh will be decommissioned. The proposed access arrangement will provide significant road safety benefits to the surrounding road network.

2.5 Stormwater Quantity Management

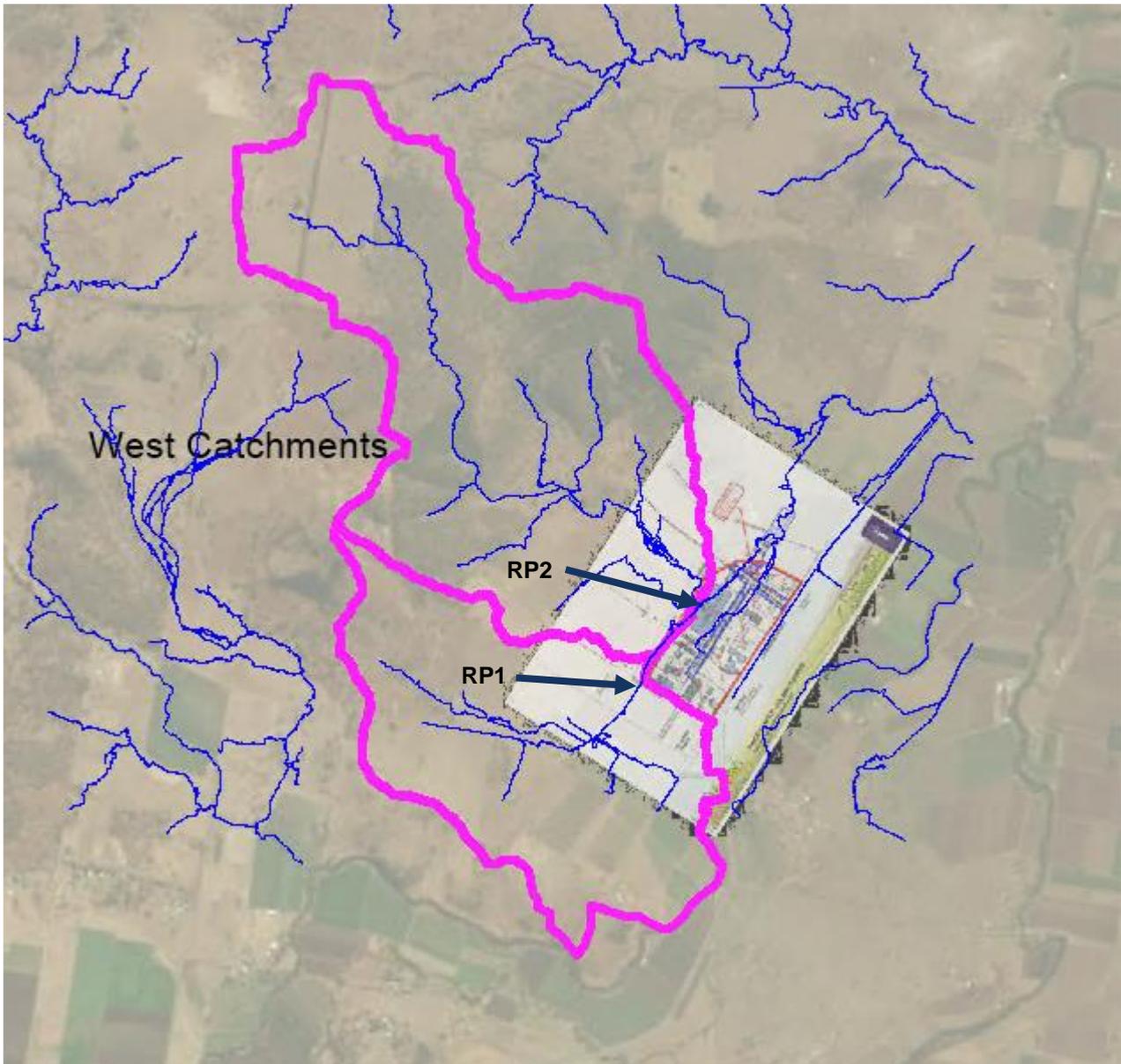
The stormwater quantity management strategies and outcomes for the proposed development are outlined by the Scenic Rim Agricultural Industrial Precinct Integrated Water Management Plan prepared by Stantec, and included in the submission under separate cover.

The objective of the stormwater management plan is to ensure that the effect of delivering the proposed development achieves a no worsening of post-development runoff compared to the runoff experience pre-development.

It has been identified that the subject site is located at the confluence of 2 No. existing stormwater catchments denoted RP1 & RP2 illustrated in Figure 2-1 below. Under existing conditions RP1 and RP2 are diverted around the existing Kalfresh facilities before discharging to Warrill Creek to the north east of the site via a series of local watercourses.

In order to achieve no worsening of stormwater discharge it is proposed that stormwater detention basins will be provided at the low point of each developed catchment to restrict runoff prior to outfall. The Integrated Water Management Plan outlines that the proposed strategy to achieve no-worsening of pre-development runoff is to detain the runoff generated from the developed site in the proposed flood conveyance channel running along the western boundary of the development pad. This conveyance channel will act as a detention basin and has largely been split into two sub-basins. These basin areas have been sized such that overall post-development peak flows discharging from the site are limited to or are less than the pre-development flows. Please refer to Section 4.1.3 of the Integrated Water Management Plan report for additional details on the proposed stormwater quantity (detention) management strategy.

Figure 2-1 Pre-Development Catchment Layout



All stormwater runoff within the site catchment shall be captured and directed into the stormwater detention basins via a mix of conventional piped network and open channel (grass swale) drainage infrastructure where the basins shall be constructed in accordance with the findings of the Integrated Water Management Plan.

The open channel or 'central road swale' has been sized to cater for peak flows from the 1 in 100 year ARI (with climate change factor allowed). The maximum water depth in the swale is 1.109m in the 1 in 10 year ARI and 1.479m in the 1 in 100 year ARI event.

The findings of the stormwater management plan recommend that the detention basins are constructed in accordance with the parameters outlined below:

Table 2-1 Proposed Detention Basin Properties

Parameter	Upstream Basin	Downstream Basin
Basin base area	285 m ² at 80.3m AHD	490 m ² at 80m AHD
Basin top area	69,440 m ² at 81.3m AHD	63,192 m ² at 80.8m AHD
Basin height	1.3m	0.8m
Total Volume at top of basin	18,324 m ³	17,250 m ³
Low flow outlet	2 x 2.4m x 0.3m box culverts	2 x 0.9m x 0.3m box culverts
High flow outlet	Access road at 81.3m AHD, acting as a weir	Northern bund at 80.8m AHD, acting as a weir

The low flow outlets (RCBC's) and high-level spillway (weir) shall be provided as per the details above. The low flow outlet will be set at the base of each detention basin allowing each basin to empty completely following a storm event. To ensure compliance with the Water Plan (Moreton) 2007 all basins will discharge down completely with no retention storage provided.

The conceptual stormwater drainage layout which includes the developed catchment areas has been provided for information only and is illustrated in Stantec sketch 510357-001-CI-1300 included in **Appendix B** of this report. The conceptual stormwater drainage layout also includes and shows the water quality outlets coordinates and legal point of discharge.

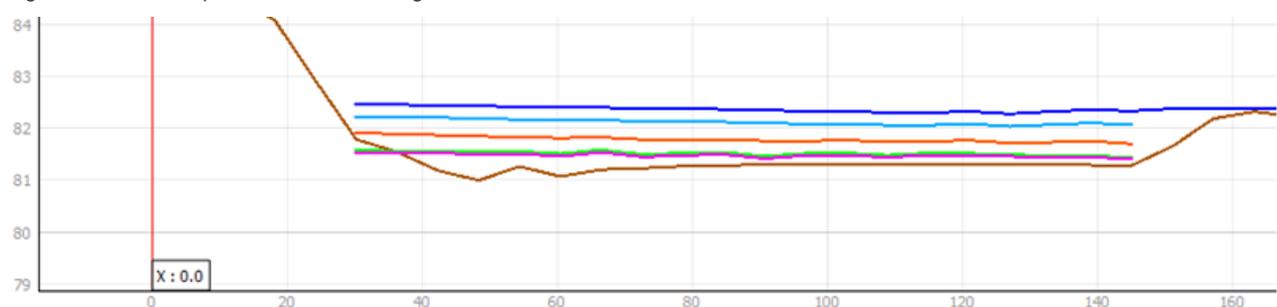
2.5.2 Weir Crossing Flood Characteristics

The flood events have also been modelled to simulate the flood height and velocity over the proposed stormwater weir crossing (Upstream Basin outlet) which provides road access to the north-west areas of the site. The summary of these results can be seen in Table 2-3 below. Additionally, the flood height can be seen below in figure 2-2 on a sample profile below for each of the respective storm events.

Table 2-2 Flood Event Heights and DV over Weir

Event	Flood Height (mAHD) Mean	DV (mean) m ² /s
100yCC	82.332	2.76
100Y	82.081	2.03
50Y	81.731	1.094
20Y	81.439	0.236
10Y	81.400	0.148

Figure 2-2 Sample Profile of Flow Heights over Weir



As shown, the weir overtops by 148mm during the 1 in 10 year ARI with a DV (depth velocity product) of 0.148. As per QUDM Table 7.4.3, a DV <0.3 m²/s is considered safe to traverse. The depth of overtopping (and associated DV) in events exceeding a 1 in 20 ARI are **not** considered trafficable.

2.6 Stormwater Quality Management

The stormwater quality management strategies and outcomes for the proposed development are outlined by the Scenic Rim Agricultural Industrial Precinct Integrated Water Management Plan, included under Appendix L of the submission.

A stormwater management strategy has been prepared outlining the water treatment measures required to be implemented in order to treat stormwater runoff from the development and achieve the stormwater quality objectives required by State and Council Planning Policies.

The proposed stormwater management strategy comprises the construction of bio-retention systems to treat stormwater runoff so that the overall pollutant load reduction meets the individual pollutant load reduction target. Each allotment will be treated on an individual basis via a bio-retention system installed prior to the discharge point of each lot. The use of proprietary stormwater quality systems may also be utilised in lieu of a bio-basin subject to planning approval.

MUSIC modelling has been completed for the internal road network in order to verify that the proposed treatment trains and bio-retention basin parameters will achieve the required water quality objectives. Table 2-2 below outlined the proposed bio-retention basin properties.

Table 2-3 Proposed Bio-Retention Basin Properties

Parameter	Bio-retention Basin
Extended Detention Depth (m)	0.2
Saturated Hydraulic Conductivity (mm/hr)	200
Filter Depth (m)	0.4
Filter Area (m ²)	varies, refer Table 12 of Stantec IWMP Plan
TN Content of Filter Media (mg/kg)	400
Orthophosphate Content of Filter Media (mg/kg)	30
Total Filter Area Required	6,798 m ²

Further details of the MUSIC model are provided in the Integrated Water Management Plan report.

2.7 Sewerage Reticulation

The proposed development will operate self-sufficiently in relation to sewerage reticulation, treatment and disposal. Therefore, no agreement or planning approval will be sought from Queensland Urban Utilities.

An internal sewerage network is to be constructed from NuSewer (PE) in accordance with the provisions of the SEQ Code. Where possible all allotments within the development have been graded such that they may be serviced by sewer located within the road reserve at the front of the allotments. A pressurized vacuum sewerage system may be installed should the depth of a gravity sewerage system become un-viable.

It is proposed the wastewater flows generated within the proposed development will be discharged to the on-site wastewater treatment plant (WWTP). Details of the on-site wastewater treatment plant are provided in Precise Environmental's 'Onsite Wastewater Management Report' which is included in this application under separate cover. The storage capacity of the WWTP is 200 kL, which has been sized using the Peak Wet Weather Flow (PW_{WWF} = 5 x ADWF = 5 x 40 kL = 200 kL).

The WWTP has been designed to treat sewerage to a Class B standard as per Queensland Water Recycling Guidelines (EPA 2005) and the Public Health Regulation 2005 (QLD). Treated sewerage will then be piped to the proposed irrigation area to the north-west of the industrial precinct.

The development will have a maximum of 800 staff working for the industry supplying and distributing vegetables. Sewerage generation estimates were based on the 'AS/NZS 1547:2000 Appendix 4.2D Typical Domestic Wastewater Flow Design Allowances (Rural Factories)'. This reference states that the maximum sewage generation rate is 50 L/d per staff member. Based on maximum rate, the average dry weather flow (ADWF) is 50 x 800 = 40,000 L/d = 0.46 L/s.

The conceptual sewer reticulation layout has been provided for information only and is illustrated in Stantec sketch 510357-001-CI-SK103 included in **Appendix D**. The conceptual sewer reticulation also includes the indicative wastewater treatment plant location in addition to the proposed irrigation area.

2.8 Water Reticulation

It is proposed that the development will be serviced by two watermains, consisting of:

- A conventional potable pressure water reticulation system treated to drinking standard; and
- A recycled watermain network for industrial and/or processing uses.

It is proposed that the internal water reticulation will be supplied by existing bores on the site. Kalfresh are currently in discussions with Department of Natural Resources, Mines and Energy in relation to securing alternative high priority water sources. Stantec understands that a high priority water license has been obtained by Kalfresh from SEQ Water for an allocation from the Warrill Creek. It is proposed that water from the Warril Creek will be pumped to a water storage dam on site before being treated and reticulated internally

Ultimately, it is intended that the proposed development will operate self-sufficiently in relation to potable water. Therefore, no agreement or planning approval will be sought from Queensland Urban Utilities.

The conceptual water reticulation layout has been provided for information only and is illustrated in Stantec sketch 510309-002-CI-SK100-102 included in **Appendix D**. The conceptual water reticulation also includes the indicative location of the water treatment plant in addition to the alignment from the site to the water intake point at the Warrill Creek. The sketches also show the proposed alignment of the raw water from the internal bored and the Warrile Creek, treated water and recycled water mains.

2.9 Composting Pad

Stantec have undertaken preliminary stormwater calculations in order to size the leachate ponds and stormwater basins required to capture runoff from the compost pads.

An overview of the concept stage stormwater functional layout plan for the compost pad area is provided in **Appendix C of this report** . The composting pad area will be bunded to divert overland flows from outside the pad around the perimeter.

3 Conclusion

From the investigations carried out it is considered that this site can be serviced by the proposed internal infrastructure and that the site may be suitably engineered in accordance with the relevant Scenic Rim Regional Council and Queensland State Government Planning Scheme policies, standards and guidelines to achieve the proposed development detailed by the RPS Plan of Subdivision.

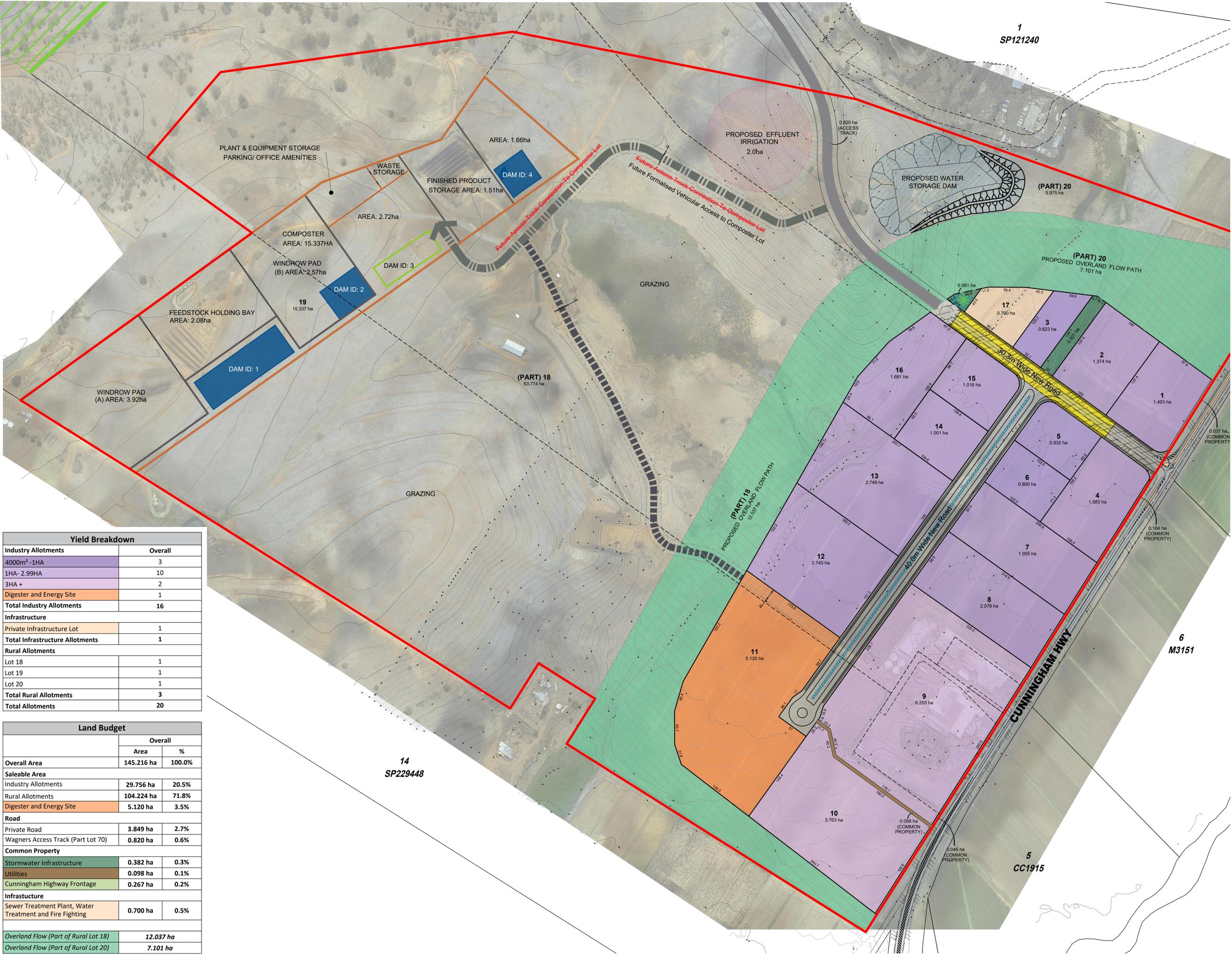
Stantec therefore consider that the site is suitable for development in accordance with the proposed Development Approval application.

APPENDIX

A

PLAN OF SRAIP

- Legend**
- Site Boundary
 - Existing Contours (1 metre)
 - Existing Boundaries
 - - - Existing Easement
 - Proposed Stormwater Infrastructure (Common Property)
 - Proposed Overland Flow (Easements, Part of Lot 18 and Lot 20)
 - Proposed Cunningham Highway Frontage Common Property (3 metres wide)
 - Proposed Utilities Common Property (4 metres wide)
 - Proposed Bio Basin
 - Proposed Water Storage Dam
 - Proposed Effluent Irrigation
 - Proposed Composting Lot
 - Lechate Pond
 - Proposed Formalised Vehicular Access
 - Proposed Plant & Equipment
 - Proposed Windrow & Finished Product
 - Proposed Stormwater Basin
 - Proposed Wagner Quarry Access - (not part of the SRAIP proposal and subject to separate development approval)
 - Significant Vegetation Area
 - Existing Vehicular Access to Composting Area
 - Access Easement for Wagners Road Alignment
 - Access Easement to Lot 19 (Composter) within Lot 11 (Digester)
 - Swale
 - Volumetric Lot - Lot 80 (1 metre below ground surface)



Yield Breakdown	
Industry Allotments	Overall
4000m ² -1HA	3
1HA- 2.99HA	10
3HA +	2
Digester and Energy Site	1
Total Industry Allotments	16
Infrastructure	
Private Infrastructure Lot	1
Total Infrastructure Allotments	1
Rural Allotments	
Lot 18	1
Lot 19	1
Lot 20	1
Total Rural Allotments	3
Total Allotments	20

Land Budget		
	Overall	
	Area	%
Overall Area	145.216 ha	100.0%
Saleable Area		
Industry Allotments	29.756 ha	20.5%
Rural Allotments	104.224 ha	71.8%
Digester and Energy Site	5.120 ha	3.5%
Road		
Private Road	3.849 ha	2.7%
Wagners Access Track (Part Lot 70)	0.820 ha	0.6%
Common Property		
Stormwater Infrastructure	0.382 ha	0.3%
Utilities	0.098 ha	0.1%
Cunningham Highway Frontage	0.267 ha	0.2%
Infrastructure		
Sewer Treatment Plant, Water Treatment and Fire Fighting	0.700 ha	0.5%
Overland Flow (Part of Rural Lot 18)	12.037 ha	
Overland Flow (Part of Rural Lot 20)	7.101 ha	

Note:
 All Lot Numbers, Dimensions and Areas are approximate only, and are subject to survey and Council approval.
 Dimensions have been rounded to the nearest 0.1 metres.
 Areas have been rounded down to the nearest 5m².
 The boundaries shown on this plan should not be used for final detailed engineers design.

Source Information:
 Site boundaries: DCOB
 Adjoining Information: DCOB
 Contours: RPS Survey
 Aerial photography: RPS Survey
 Overland Flow Path: Aurecon

URBAN DESIGN
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APPENDIX

B

PRELIM ENGINEERING SKETCHES



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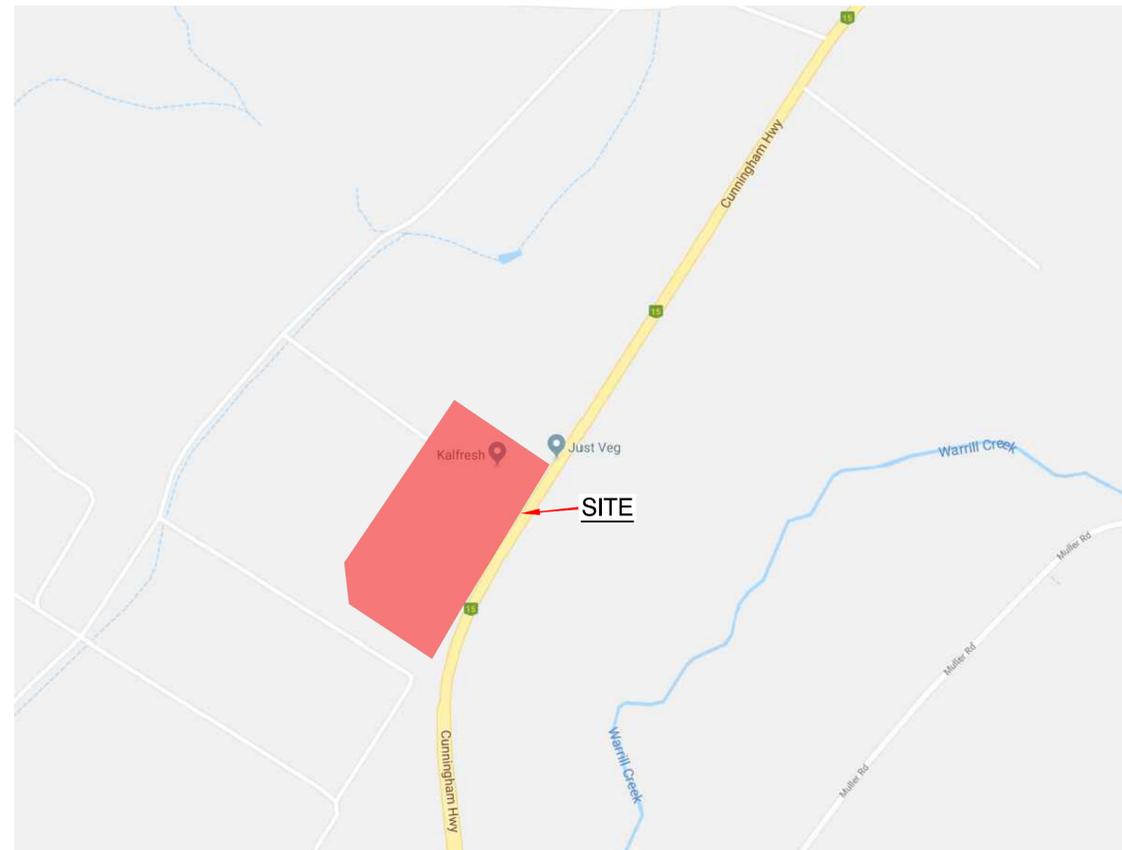
KALFRESH PTY LTD
SCENIC RIM
AGRICULTURAL INDUSTRIAL PRECINCT

DATE PLOTTED: 22 February, 2023 5:55 PM BY: PAULIO ZOTTIS



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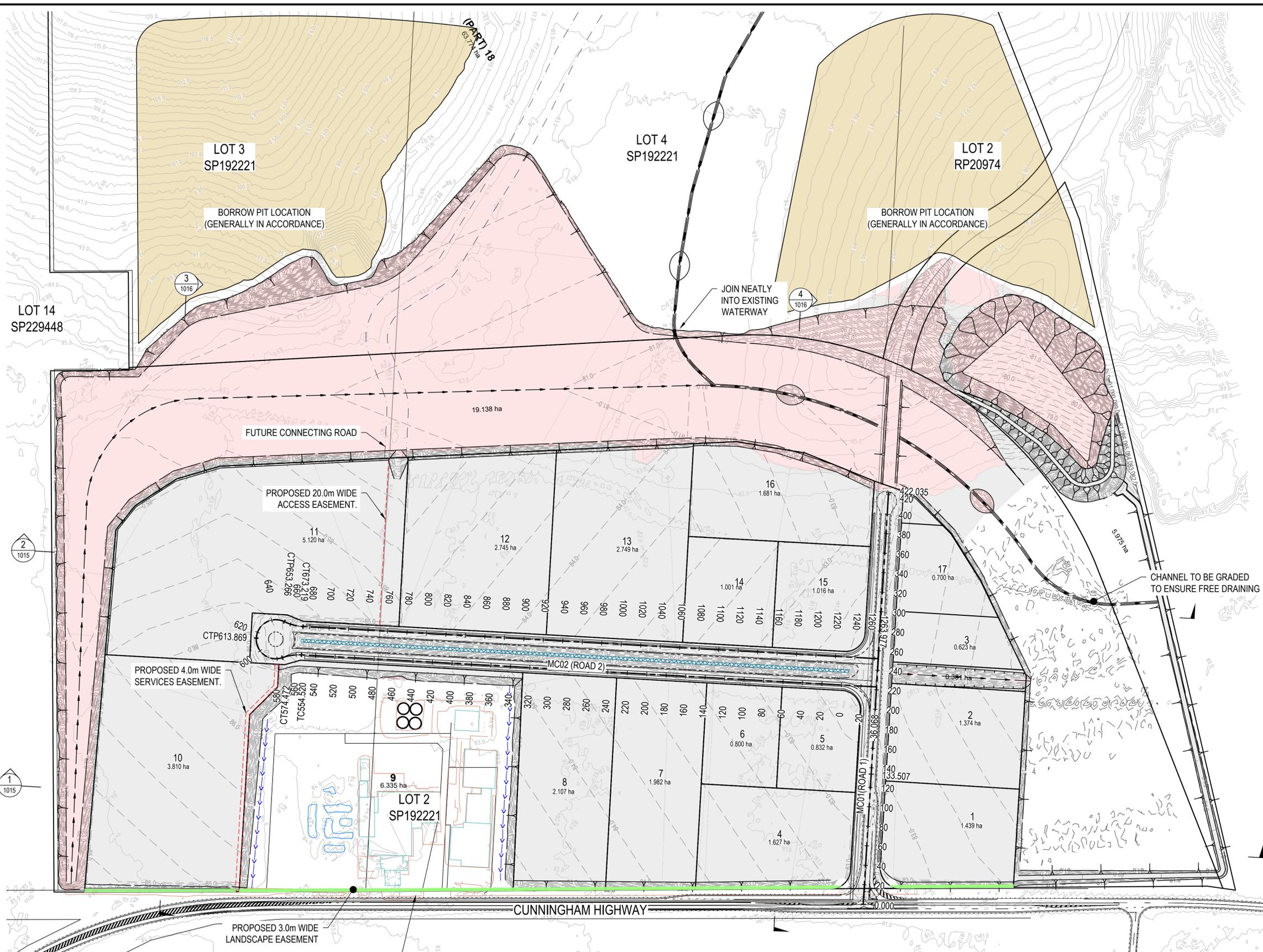
SCHEDULE OF DRAWINGS	
DRAWING No.	DESCRIPTION
510357-001-CI-1000	COVER SHEET
GENERAL	
510357-001-CI-1001	DRAWING SCHEDULE AND LOCALITY PLAN
510357-001-CI-1010	BULK EARTHWORKS LAYOUT PLAN
510357-001-CI-1011	ROAD TYPICAL CROSS SECTIONS
510357-001-CI-1015	BULK EARTHWORKS SECTIONS SHEET 1
510357-001-CI-1016	BULK EARTHWORKS SECTIONS SHEET 2
ROADWORKS	
510357-001-CI-1100	MC01 LONGITUDINAL SECTION
510357-001-CI-1101	MC02 LONGITUDINAL SECTION - SHEET 01
510357-001-CI-1102	MC02 LONGITUDINAL SECTION - SHEET 02
STORMWATER DRAINAGE	
510357-001-CI-1300	STORMWATER LAYOUT PLAN
SEWER RETICULATION	
510357-001-CI-1500	SEWER RETICULATION LAYOUT PLAN
WATER RETICULATION	
510357-001-CI-1600	WATER RETICULATION LAYOUT PLAN



LOCALITY PLAN
NOT TO SCALE

XREFs: CAD File: \\austfiles02\cadd\510357\001 - CIVIL ENGINEERING\Drawings\510357-001-CI-1001.dwg

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		DRAWING SCHEDULE AND LOCALITY PLAN	
		Datum AHD	Date 3/12/2019
Drawing Number 510357-001-CI-1001			Revision 3



EARTHWORKS
 DATE: FRIDAY JAN 20TH 17:00:00 2023
 REPORT FILE: L:\15103-57\008 - DETAILED BULK EARTHWORKS\12D\20230125 DESIGN OPT 2 REPORT.rpt

VOLUMES FROM TIN "S EXIST" TO TIN "DESIGN OPT 2"
 CUT VOLUMES ARE NEGATIVE
 FILL VOLUMES ARE POSITIVE

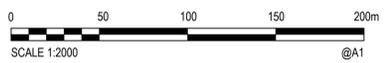
TOTAL CUT	-488593.342m ³
TOTAL FILL	709025.932m ³
IE EXCESS OF FILL OVER CUT	220432.590m ³

NOTE:
 1. INDICATED EARTHWORKS VOLUMES ARE NETT AND EXCLUDE BULKING / COMPACTION FACTORS.
 2. SPOIL LOCATION FOR EXCESS CUT MATERIAL TO BE ADVISED BY THE SUPERINTENDENT.
 3. EARTHWORKS ARE STILL WORKS IN PROGRESS. TO BE CONFIRMED

LEGEND

PROPOSED PROPERTY BOUNDARY	—
PROPOSED NOMINAL KERB LINE	- - -
ROAD CENTRELINE	—+—
PROPOSED CUT	█ (Pink)
PROPOSED FILL	█ (Grey)
PROPOSED LANDSCAPE EASEMENT	█ (Green)
PROPOSED TOP OF BATTER	—+—
PROPOSED BOTTOM OF BATTER	—+—
EXISTING OVERLAND FLOWPATH	—+—
PROPOSED OVERLAND FLOWPATH	→ → →
PROPOSED CONTOURS (0.25m)	- - - 36.5
EXISTING CONTOURS (0.5m)	- - - 36.5

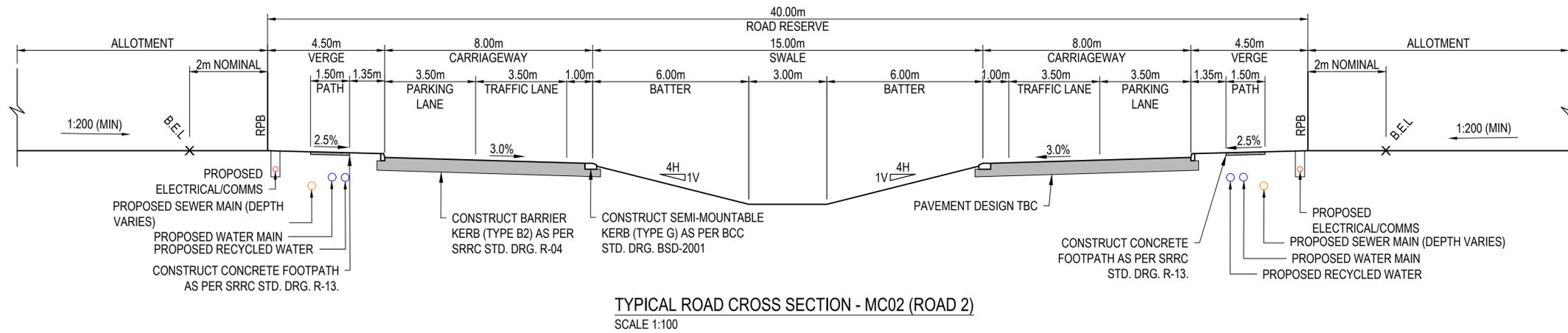
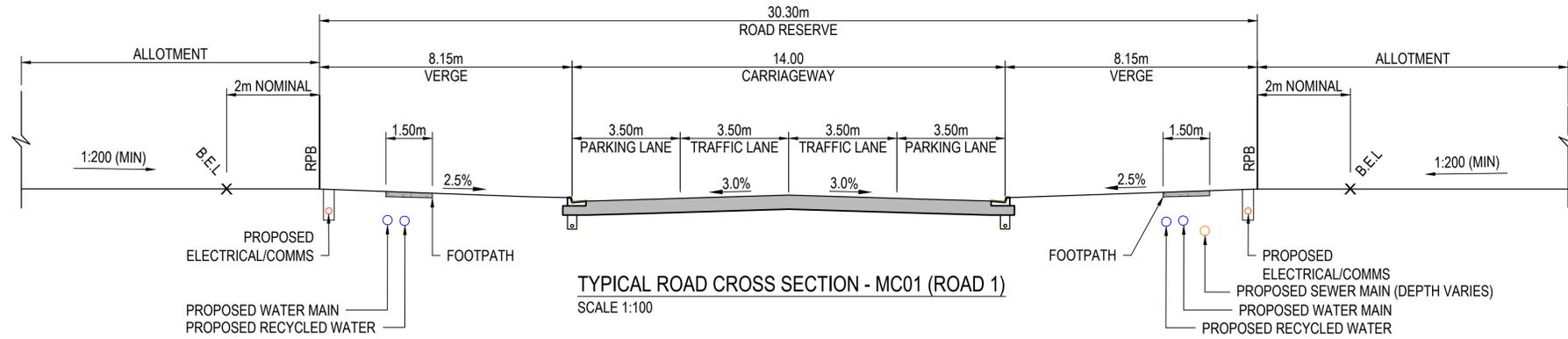
EARTHWORKS PLAN
SCALE 1:2000



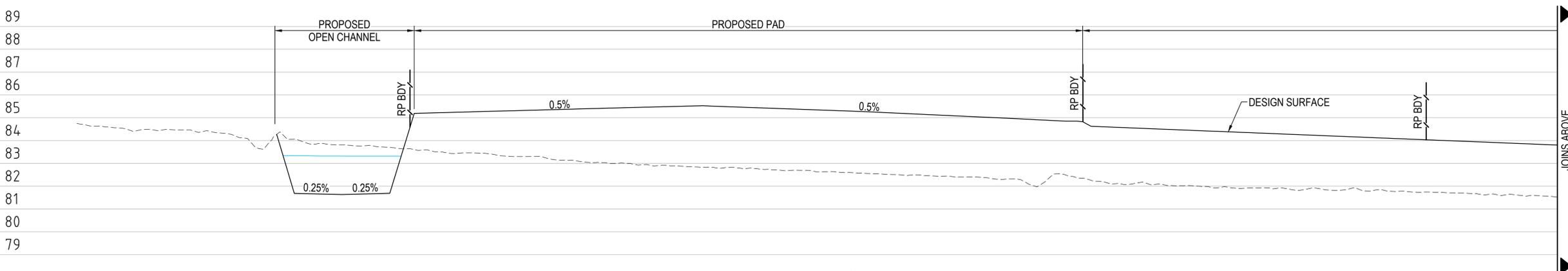
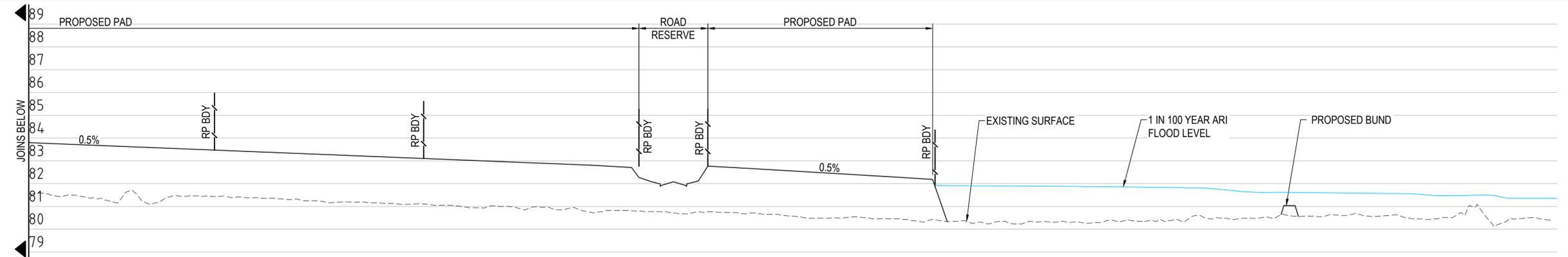
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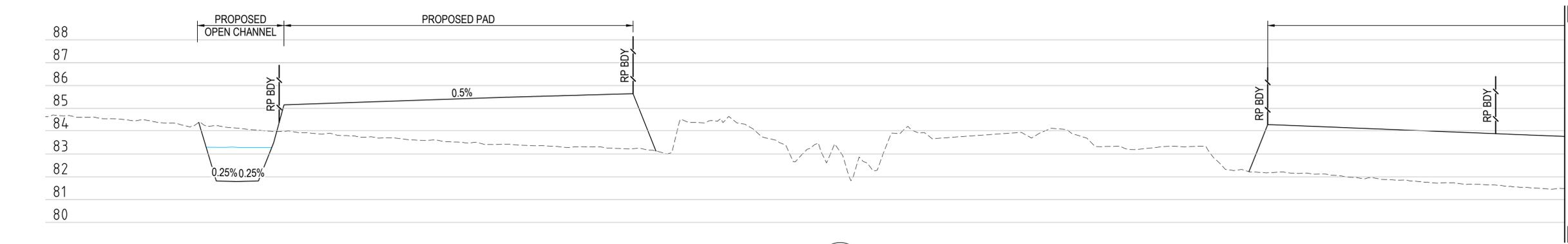
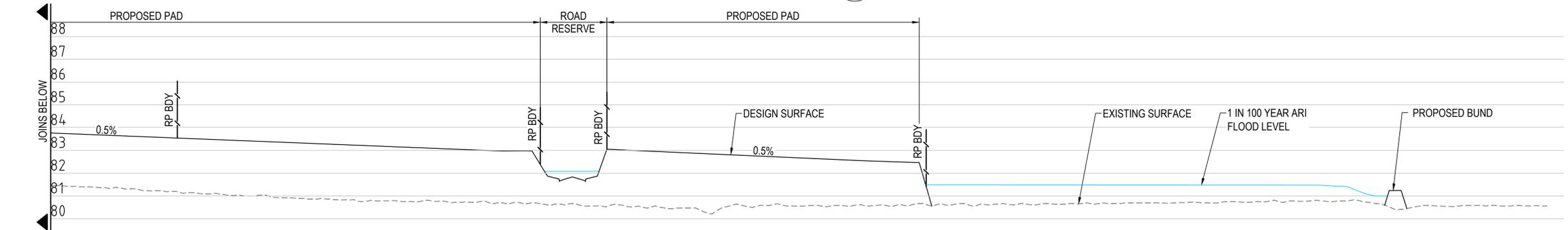
KALFRESH PTY LTD			
BULK EARTHWORKS LAYOUT PLAN			
Datum	Date	Scale	Size
AHD	3/12/2019	AS SHOWN	A1
Drawing Number	Revision		
510357-001-CI-1010			4



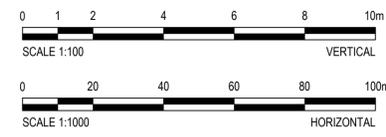
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<p>ROAD TYPICAL CROSS SECTIONS</p> <table border="1"> <tr> <th>Datum</th> <th>Date</th> <th>Scale</th> <th>Size</th> </tr> <tr> <td>AHD</td> <td>3/12/2019</td> <td>AS SHOWN</td> <td>A1</td> </tr> </table>			Datum	Date	Scale	Size	AHD	3/12/2019	AS SHOWN	A1	<table border="1"> <tr> <td>Drawing Number</td> <td>Revision</td> </tr> <tr> <td>510357-001-CI-1011</td> <td>4</td> </tr> </table>		Drawing Number	Revision	510357-001-CI-1011
Datum	Date	Scale	Size												
AHD	3/12/2019	AS SHOWN	A1												
Drawing Number	Revision														
510357-001-CI-1011	4														



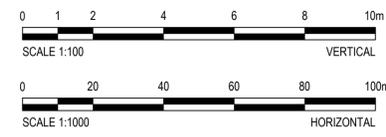
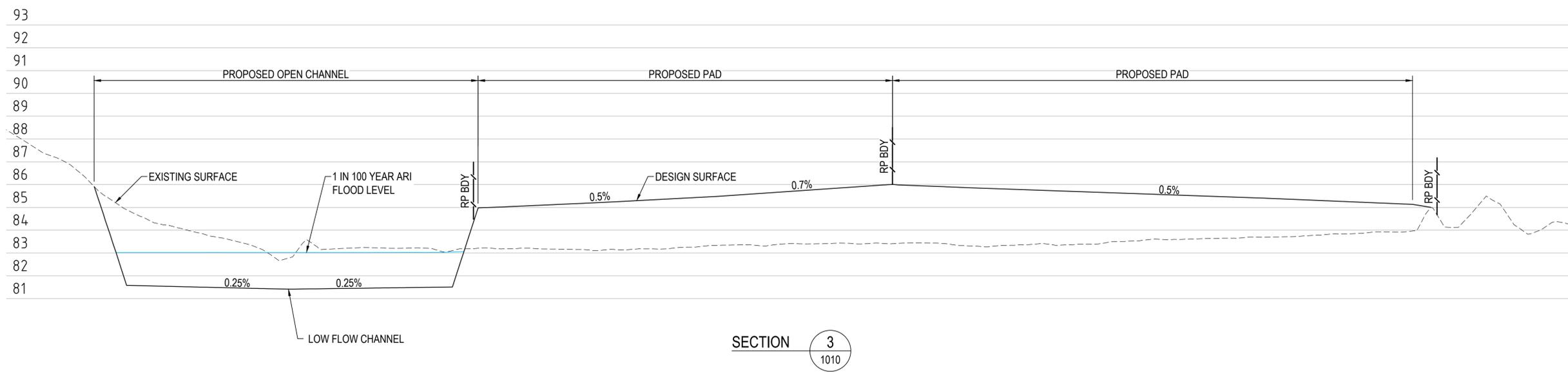
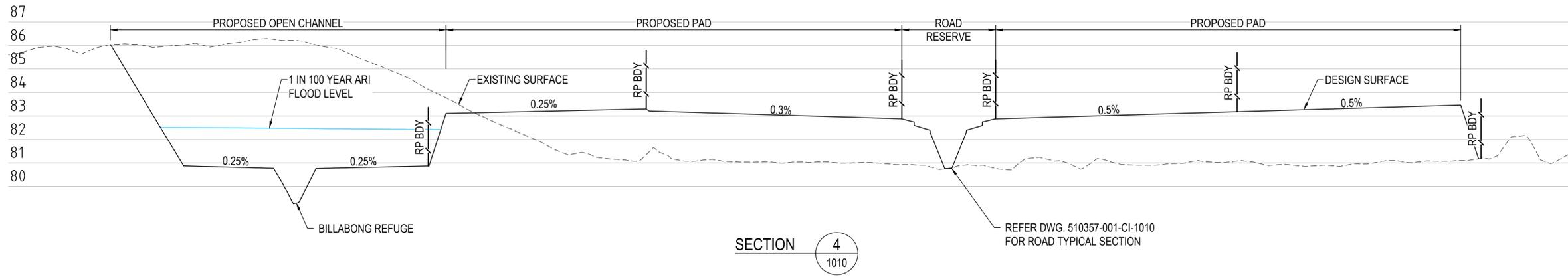
SECTION 2
1010



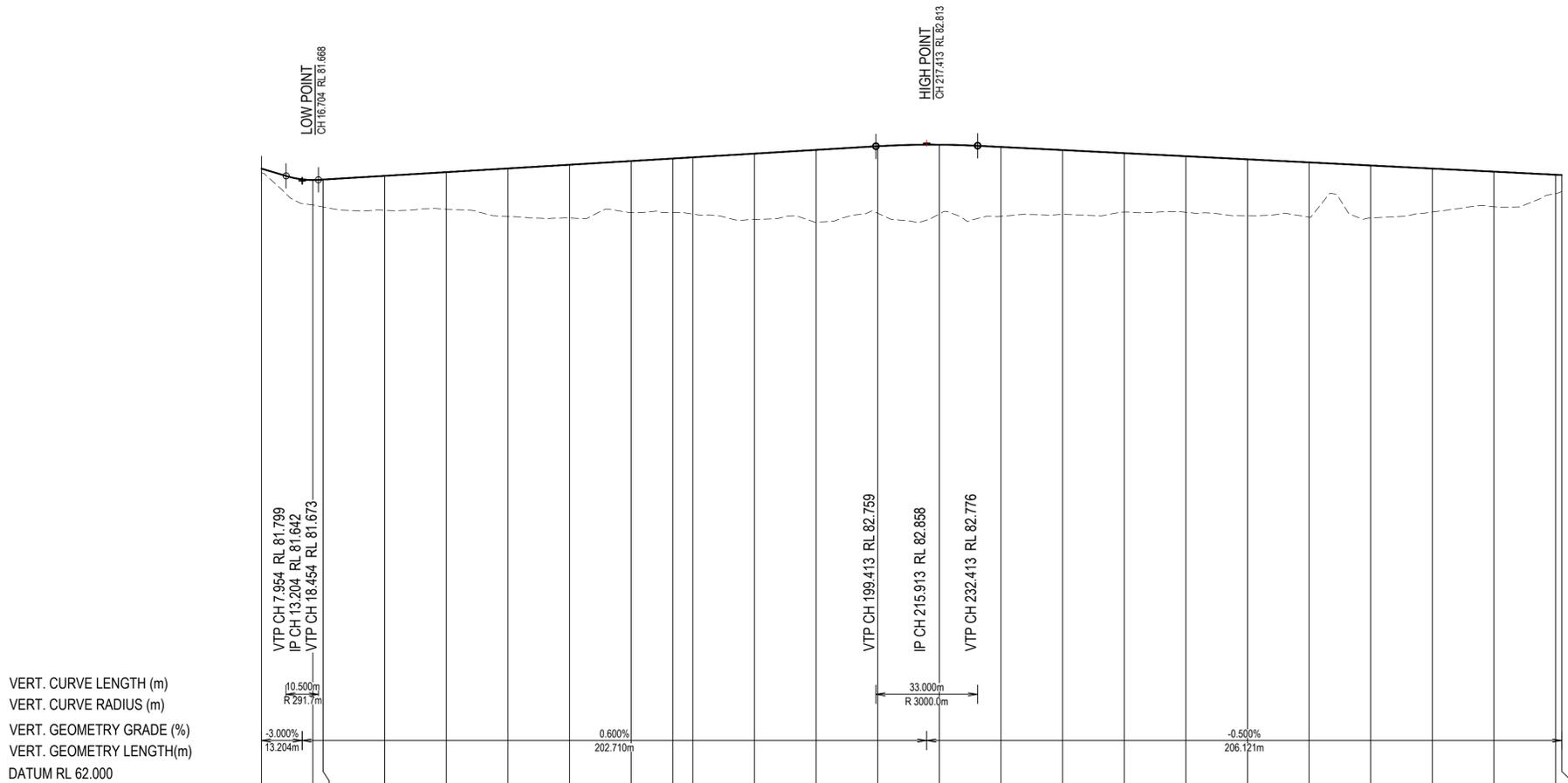
SECTION 1
1010



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<p>Datum AHD</p>	<p>Date 3/12/2019</p>		<p>Scale AS SHOWN</p>	<p>Size A1</p>
<p>Drawing Number 510357-001-CI-1015</p>			<p>Revision 4</p>	



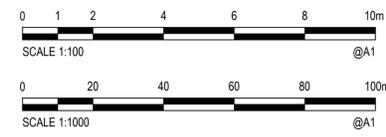
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<p>Datum: AHD</p>	<p>Date: 3/12/2019</p>		<p>Scale: AS SHOWN</p>	<p>Size: A1</p>
<p>Drawing Number: 510357-001-CI-1016</p>			<p>Revision: 4</p>	



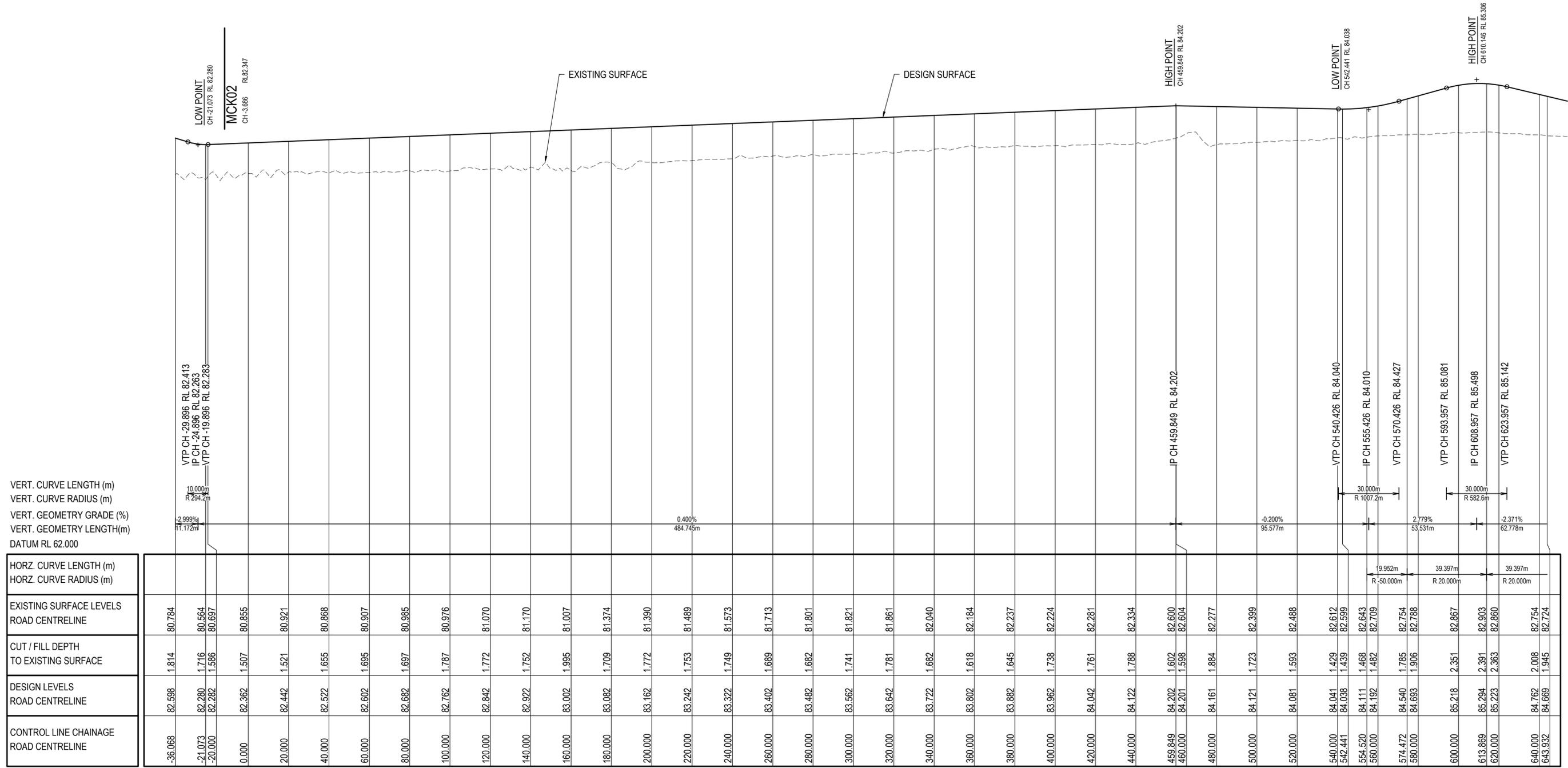
VERT. CURVE LENGTH (m)
 VERT. CURVE RADIUS (m)
 VERT. GEOMETRY GRADE (%)
 VERT. GEOMETRY LENGTH(m)
 DATUM RL 62.000

HORZ. CURVE LENGTH (m)	HORZ. CURVE RADIUS (m)	EXISTING SURFACE LEVELS ROAD CENTRELINE	CUT / FILL DEPTH TO EXISTING SURFACE	DESIGN LEVELS ROAD CENTRELINE	CONTROL LINE CHAINAGE ROAD CENTRELINE
		82.038	0.004	81.884	0.000
		81.668	0.663	80.855	16.704
		81.663	0.733	80.793	20.000
		81.803	0.940	80.686	40.000
		81.923	1.009	80.717	60.000
		82.043	1.337	80.489	80.000
		82.163	1.488	80.437	100.000
		82.283	1.414	80.611	120.000
		82.364	1.481	80.612	133.507
		82.403	1.561	80.565	140.000
		82.523	1.838	80.387	160.000
		82.643	2.032	80.293	180.000
		82.763	1.833	80.592	200.000
		82.812	1.885	80.570	220.000
		82.738	1.868	80.492	240.000
		82.638	1.682	80.559	260.000
		82.538	1.492	80.628	280.000
		82.438	1.380	80.620	300.000
		82.338	1.369	80.511	320.000
		82.238	1.299	80.466	340.000
		82.138	0.719	80.436	360.000
		82.038	1.335	80.628	380.000
		81.938	1.305	80.798	400.000
		81.838	1.013	81.230	420.000
		81.828	0.962	81.296	422.035

MC01 - ROAD 1
 SCALE: H 1:1000
 V 1:100



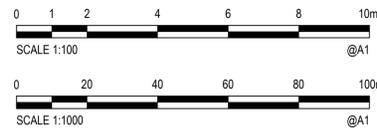
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Datum AHD	Date 3/12/2019		Scale AS SHOWN	Size A1
Drawing Number 510357-001-CI-1100			Revision 3	



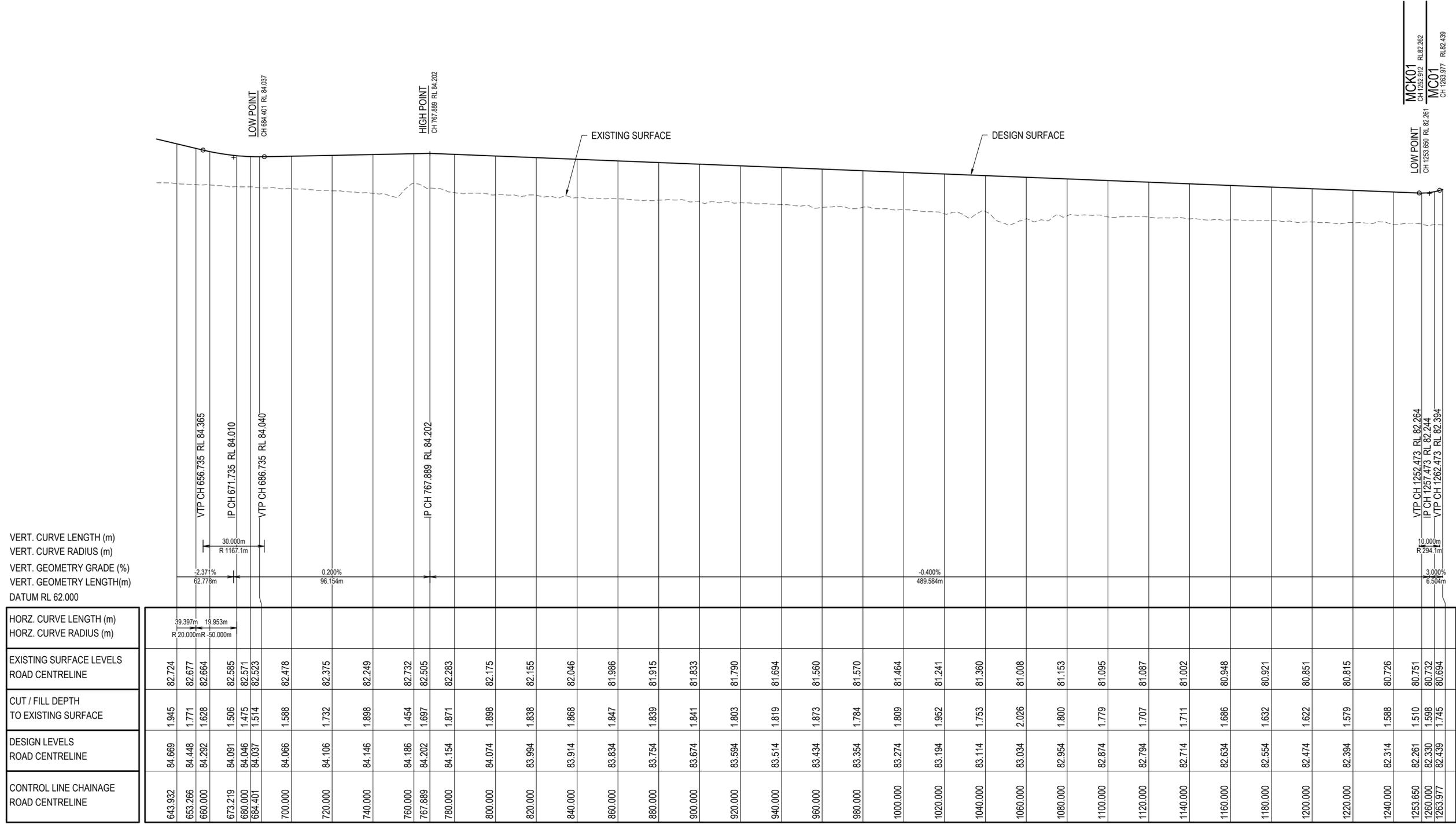
VERT. CURVE LENGTH (m)
 VERT. CURVE RADIUS (m)
 VERT. GEOMETRY GRADE (%)
 VERT. GEOMETRY LENGTH(m)
 DATUM RL 62.000

HORZ. CURVE LENGTH (m)	HORZ. CURVE RADIUS (m)	EXISTING SURFACE LEVELS ROAD CENTRELINE	CUT / FILL DEPTH TO EXISTING SURFACE	DESIGN LEVELS ROAD CENTRELINE	CONTROL LINE CHAINAGE ROAD CENTRELINE
36.068		80.784	1.814	82.598	-36.068
		80.564	1.716	82.280	-21.073
		80.697	1.586	82.282	-20.000
		80.855	1.507	82.362	0.000
		80.921	1.521	82.442	20.000
		80.868	1.655	82.522	40.000
		80.907	1.695	82.602	60.000
		80.985	1.697	82.682	80.000
		80.976	1.787	82.762	100.000
		81.070	1.772	82.842	120.000
		81.170	1.752	82.922	140.000
		81.007	1.995	83.002	160.000
		81.374	1.709	83.082	180.000
		81.390	1.772	83.162	200.000
		81.489	1.753	83.242	220.000
		81.573	1.749	83.322	240.000
		81.713	1.689	83.402	260.000
		81.801	1.682	83.482	280.000
		81.821	1.741	83.562	300.000
		81.861	1.781	83.642	320.000
		82.040	1.682	83.722	340.000
		82.184	1.618	83.802	360.000
		82.237	1.645	83.882	380.000
		82.224	1.738	83.962	400.000
		82.281	1.761	84.042	420.000
		82.334	1.788	84.122	440.000
		82.600	1.602	84.202	459.849
		82.604	1.598	84.201	460.000
		82.277	1.884	84.161	480.000
		82.399	1.723	84.121	500.000
		82.488	1.593	84.081	520.000
		82.599	1.429	84.041	540.000
		82.599	1.439	84.038	542.441
		82.643	1.468	84.111	554.520
		82.709	1.462	84.192	560.000
		82.754	1.785	84.540	574.472
		82.788	1.906	84.683	580.000
		82.867	2.351	85.218	600.000
		82.903	2.391	85.294	613.869
		82.860	2.363	85.223	620.000
		82.754	2.008	84.762	640.000
		82.724	1.945	84.689	643.932

MC02 - ROAD 2
 SCALE: H 1:1000
 V 1:100

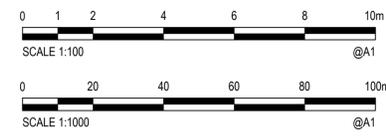


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HORZ. CURVE LENGTH (m) HORZ. CURVE RADIUS (m)	EXISTING SURFACE LEVELS ROAD CENTRELINE	CUT / FILL DEPTH TO EXISTING SURFACE	DESIGN LEVELS ROAD CENTRELINE	CONTROL LINE CHAINAGE ROAD CENTRELINE
643.932	84.669	1.945	84.669	643.932
653.266	84.448	1.771	84.448	653.266
660.000	84.292	1.628	84.292	660.000
673.219	84.091	1.506	84.091	673.219
680.000	84.046	1.475	84.046	680.000
684.401	84.037	1.514	84.037	684.401
700.000	84.066	1.588	84.066	700.000
720.000	84.106	1.732	84.106	720.000
740.000	84.146	1.898	84.146	740.000
760.000	84.186	1.454	84.186	760.000
767.889	84.202	1.697	84.202	767.889
780.000	84.154	1.871	84.154	780.000
800.000	84.074	1.898	84.074	800.000
820.000	83.994	1.838	83.994	820.000
840.000	83.914	1.868	83.914	840.000
860.000	83.834	1.847	83.834	860.000
880.000	83.754	1.839	83.754	880.000
900.000	83.674	1.841	83.674	900.000
920.000	83.594	1.803	83.594	920.000
940.000	83.514	1.819	83.514	940.000
960.000	83.434	1.873	83.434	960.000
980.000	83.354	1.784	83.354	980.000
1000.000	83.274	1.809	83.274	1000.000
1020.000	83.194	1.952	83.194	1020.000
1040.000	83.114	1.753	83.114	1040.000
1060.000	83.034	2.026	83.034	1060.000
1080.000	82.954	1.800	82.954	1080.000
1100.000	82.874	1.779	82.874	1100.000
1120.000	82.794	1.707	82.794	1120.000
1140.000	82.714	1.711	82.714	1140.000
1160.000	82.634	1.686	82.634	1160.000
1180.000	82.554	1.632	82.554	1180.000
1200.000	82.474	1.622	82.474	1200.000
1220.000	82.394	1.579	82.394	1220.000
1240.000	82.314	1.588	82.314	1240.000
1253.650	82.261	1.510	82.261	1253.650
1260.000	82.330	1.598	82.330	1260.000
1263.977	82.439	1.745	82.439	1263.977

MC02 - ROAD 2
 SCALE: H 1:1000
 V 1:100



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KALFRESH PTY LTD
 MC02 LONGITUDINAL SECTION - SHEET 02

Datum	Date	Scale	Size
AHD	3/12/2019	AS SHOWN	A1
Drawing Number	Revision		
510357-001-CI-1102			3



LEGEND

PROPOSED PROPERTY BOUNDARY	—
EXISTING PROPERTY BOUNDARY	—
PROPOSED SEWER MAIN	— S — S —
PROPOSED NOMINAL KERB LINE	— · — · —
PROPOSED CONTOURS (0.25m)	- - - - - 36.5

SEWER RETICULATION PLAN
SCALE 1:2000



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KALFRESH PTY LTD			
SEWER RETICULATION LAYOUT PLAN			
Datum	Date	Scale	Size
AHD	3/12/2019	AS SHOWN	A1
Drawing Number			Revision
510357-001-CI-1500			4



LEGEND

STAGE BOUNDARY	—
EXISTING PROPERTY BOUNDARY	—
PROPOSED WATER MAIN	— W —
PROPOSED RECYCLE WATER MAIN	— RW —
PROPOSED WATER METER	— X —
EXISTING WATER	— W —
EXISTING STORMWATER	— SWD —
EXISTING ELECTRICAL/TELSTRA/GAS	— E/T/G —
PROPOSED CONTOURS (0.25m)	--- 36.5 ---

WATER RETICULATION PLAN
 SCALE 1:2000

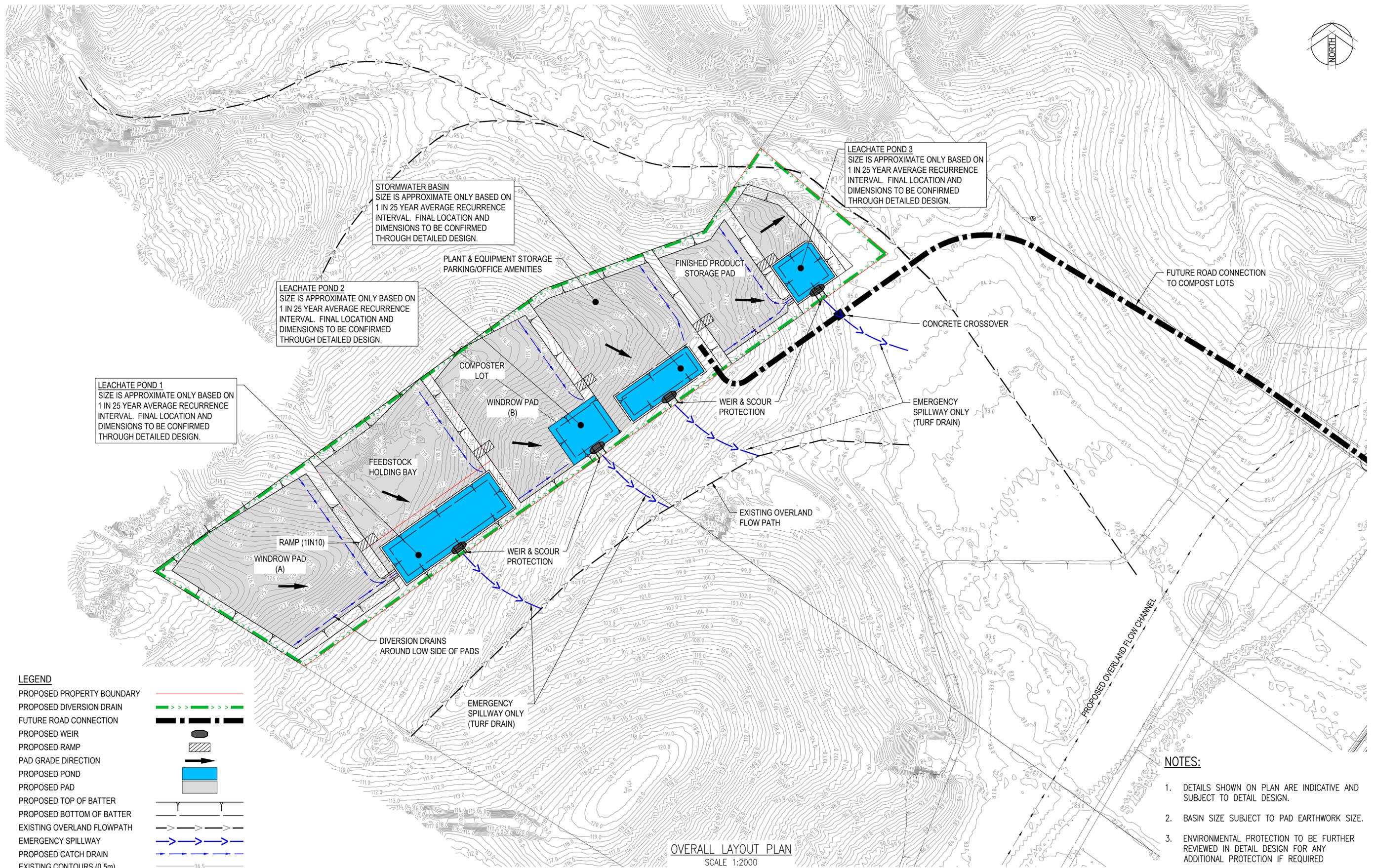


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		<p>WATER RETICULATION LAYOUT PLAN</p>			
		<p>Datum: AHD</p>	<p>Date: 3/12/2019</p>	<p>Scale: AS SHOWN</p>	<p>Size: A1</p>
		<p>Drawing Number: 510357-001-CI-1600</p>		<p>Revision: 3</p>	

APPENDIX

C

COMPOST PAD STORMWATER
LAYOUT



LEACHATE POND 1
 SIZE IS APPROXIMATE ONLY BASED ON
 1 IN 25 YEAR AVERAGE RECURRENCE
 INTERVAL. FINAL LOCATION AND
 DIMENSIONS TO BE CONFIRMED
 THROUGH DETAILED DESIGN.

LEACHATE POND 2
 SIZE IS APPROXIMATE ONLY BASED ON
 1 IN 25 YEAR AVERAGE RECURRENCE
 INTERVAL. FINAL LOCATION AND
 DIMENSIONS TO BE CONFIRMED
 THROUGH DETAILED DESIGN.

LEACHATE POND 3
 SIZE IS APPROXIMATE ONLY BASED ON
 1 IN 25 YEAR AVERAGE RECURRENCE
 INTERVAL. FINAL LOCATION AND
 DIMENSIONS TO BE CONFIRMED
 THROUGH DETAILED DESIGN.

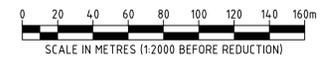
LEGEND

PROPOSED PROPERTY BOUNDARY	
PROPOSED DIVERSION DRAIN	
FUTURE ROAD CONNECTION	
PROPOSED WEIR	
PROPOSED RAMP	
PAD GRADE DIRECTION	
PROPOSED POND	
PROPOSED PAD	
PROPOSED TOP OF BATTER	
PROPOSED BOTTOM OF BATTER	
EXISTING OVERLAND FLOWPATH	
EMERGENCY SPILLWAY	
PROPOSED CATCH DRAIN	
EXISTING CONTOURS (0.5m)	

- NOTES:**
1. DETAILS SHOWN ON PLAN ARE INDICATIVE AND SUBJECT TO DETAIL DESIGN.
 2. BASIN SIZE SUBJECT TO PAD EARTHWORK SIZE.
 3. ENVIRONMENTAL PROTECTION TO BE FURTHER REVIEWED IN DETAIL DESIGN FOR ANY ADDITIONAL PROTECTION IF REQUIRED

OVERALL LAYOUT PLAN
 SCALE 1:2000

NOT FOR CONSTRUCTION



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KALFRESH PTY LTD
 SCENIC RIM
 AGRICULTURAL INDUSTRIAL PRECINCT

Date 26-09-2023 Scale 1:2000 Size A1

PROPOSED COMPOSTER
 FUNCTIONAL LAYOUT PLAN

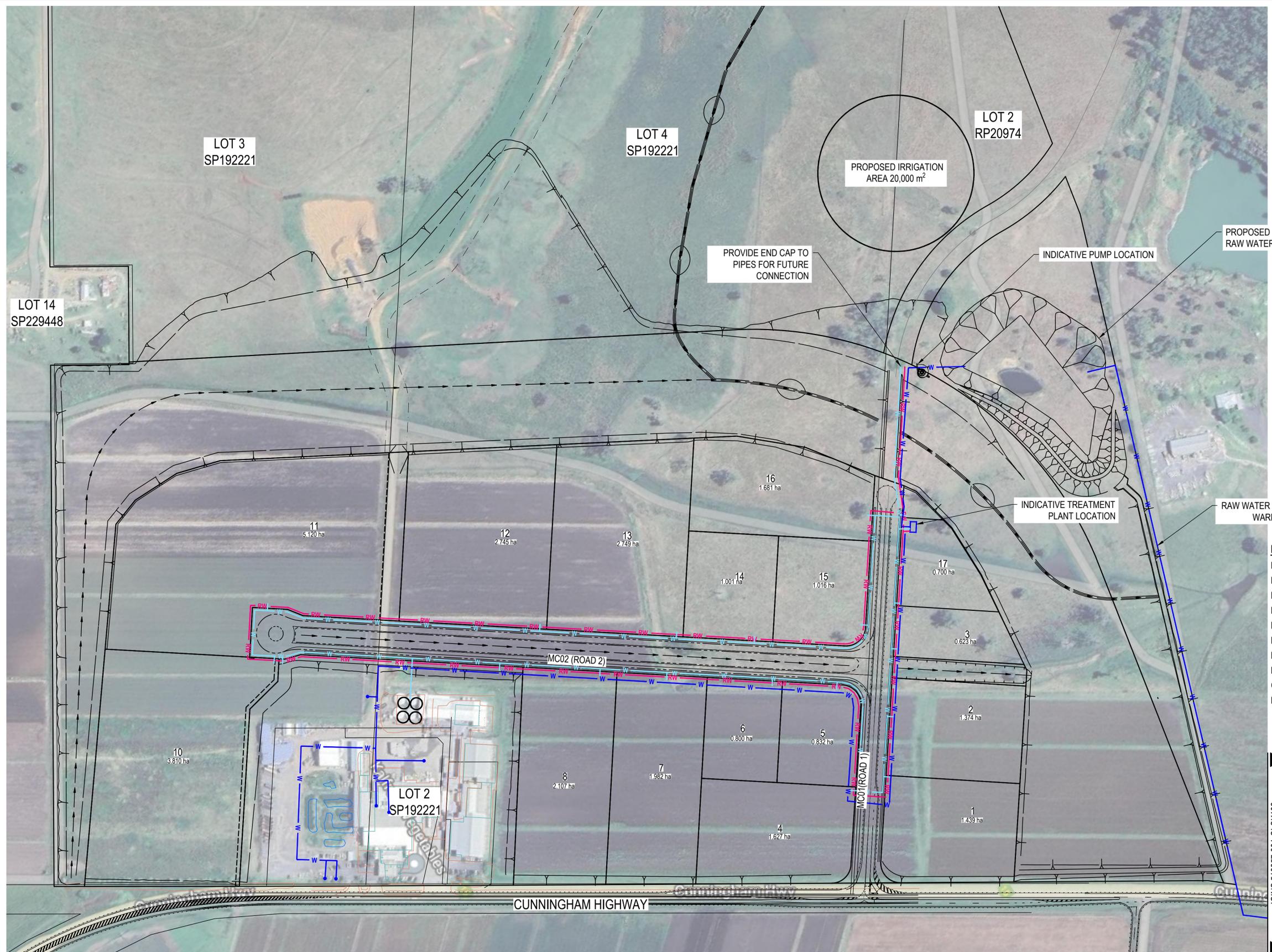
510357-007-SK001
 Drawing Number

2
 Revision

APPENDIX

D

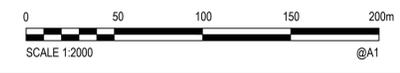
WATER AND WASTEWATER
RETICULATION SCHEMATICS



LEGEND

- PROPOSED PROPERTY BOUNDARY
- PROPOSED NOMINAL KERB LINE
- PROPOSED RAW WATER MAIN ALIGNMENT
- PROPOSED TREATED WATER MAIN
- PROPOSED RECYCLE WATER MAIN
- PROPOSED WATER METER
- PROPOSED TOP OF BATTER
- PROPOSED BOTTOM OF BATTER
- OVERLAND FLOW PATH
- PROPOSED PUMP

LAYOUT PLAN
SCALE 1:2000



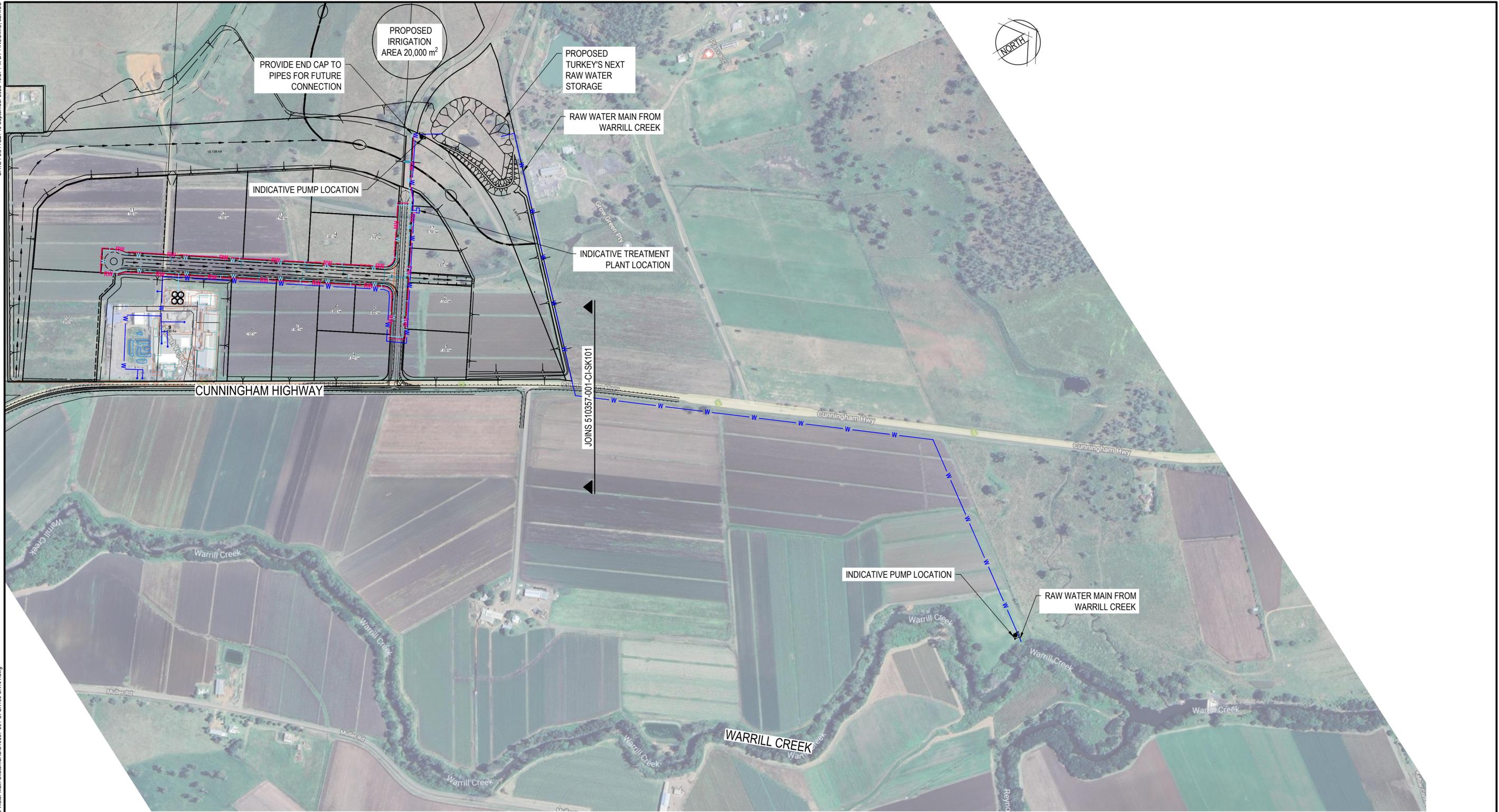
NOTE:
REFER TO DRAWING 510357-001-CI-SK101 FOR TYPICAL SERVICES CROSS SECTIONS.

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KALFRESH PTY LTD			
SITE WATER SUPPLY AND RETICULATION PLAN			
Datum	Date	Scale	Size
AHD	3/12/2019	AS SHOWN	A1
Drawing Number			Revision
510357-001-CI-SK100			1

DATE PLOTTED: 19 September 2023 10:27 PM BY: INCOMIO, JERICHO



LEGEND

PROPOSED PROPERTY BOUNDARY	— — — — —
PROPOSED NOMINAL KERB LINE	— — — — —
PROPOSED RAW WATER MAIN ALIGNMENT	— W — — —
PROPOSED TREATED WATER MAIN	— T — — —
PROPOSED RECYCLE WATER MAIN	— RW — — —
PROPOSED WATER METER	— X — — —
PROPOSED TOP OF BATTER	— T — — —
PROPOSED BOTTOM OF BATTER	— B — — —
OVERLAND FLOW PATH	— — — — —
PROPOSED PUMP	⊗

LAYOUT PLAN
SCALE 1:4000

NOTE:
REFER TO DRAWING 510357-001-CI-SK101
FOR TYPICAL SERVICES CROSS SECTIONS.



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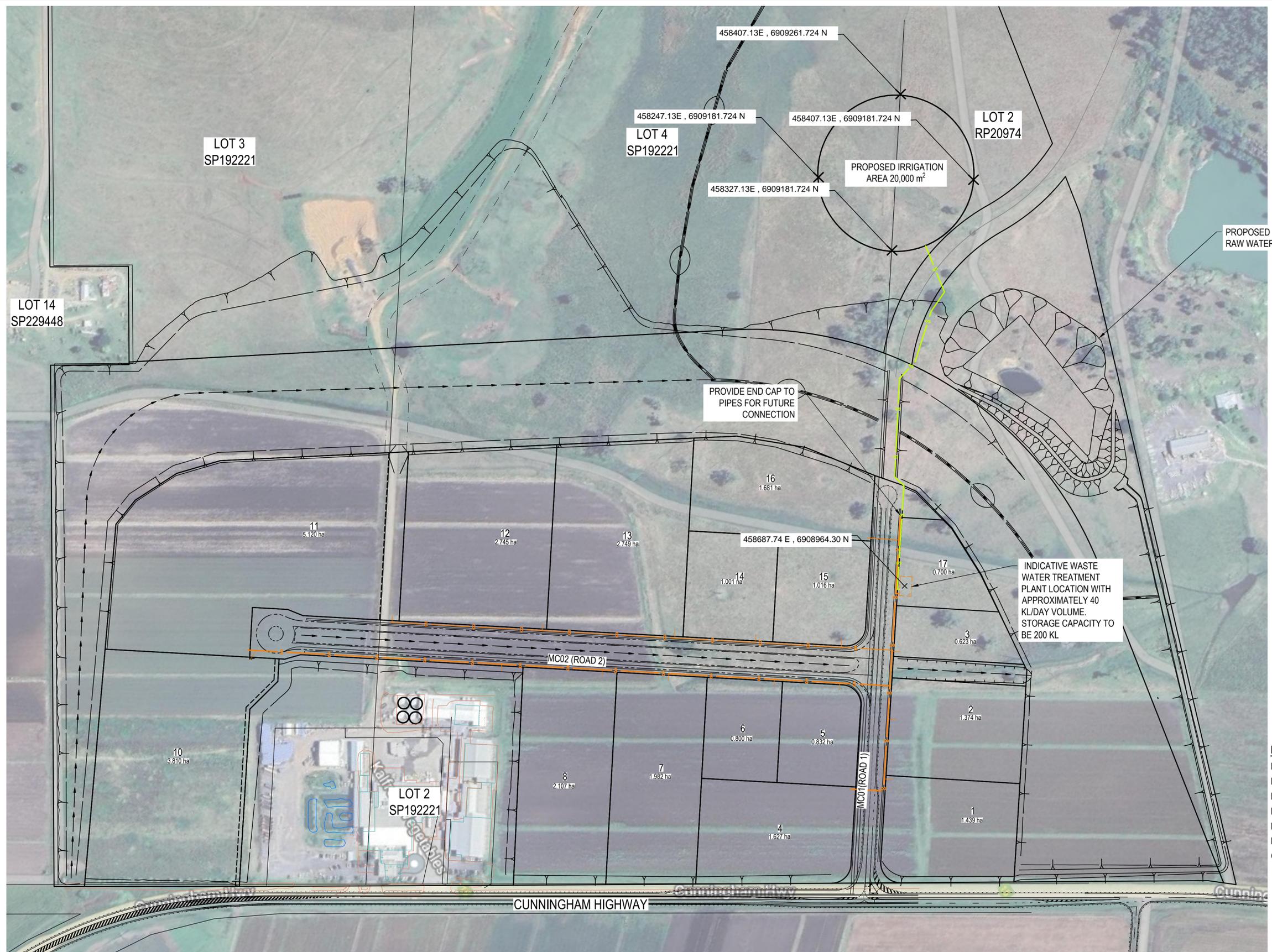
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KALFRESH PTY LTD			
SITE WATER SUPPLY AND RETICULATION PLAN			
Datum	Date	Scale	Size
AHD	3/12/2019	AS SHOWN	A1
Drawing Number			Revision
510357-001-CI-SK102			1

XREFS: XR-SURVEY; XR-CUTFILL; XR-CONT-EXIST; XR-DSGN-OP2; XR-CONT-DSGN-CP2a; CAD File: \\au2035-ppfss\shared_projects\304701259100_xxx\drawings\sketches\20230905_Water_Main_Sketch\CAD\510357-001-CI-SK100-SK101.dwg

DATE PLOTTED: 19 September 2023 10:39 PM BY: INCOMIO_JERICO

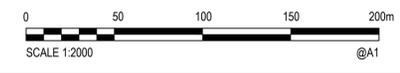
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LEGEND

PROPOSED PROPERTY BOUNDARY	
PROPOSED NOMINAL KERB LINE	
PROPOSED SEWER MAIN	
PROPOSED EFFLUENT MAIN	
PROPOSED TOP OF BATTER	
PROPOSED BOTTOM OF BATTER	
OVERLAND FLOW PATH	

LAYOUT PLAN
SCALE 1:2000



NOTE:
REFER TO DRAWING 510357-001-CI-SK101 FOR TYPICAL SERVICES CROSS SECTIONS.

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KALFRESH PTY LTD			
SITE SEWER RETICULATION PLAN			
Datum	Date	Scale	Size
AHD	3/12/2019	AS SHOWN	A1
Drawing Number			Revision
510357-001-CI-SK103			1